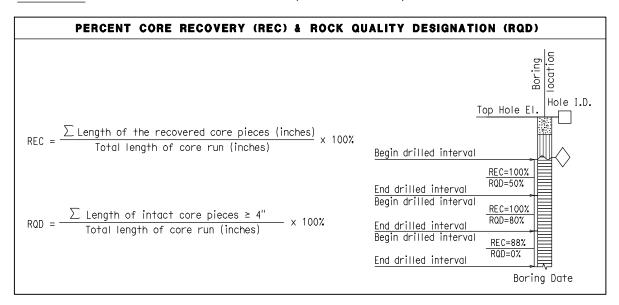
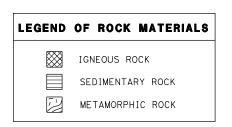
REFERENCE: CALTRANS SOIL & ROCK LOGGING, CLASSIFICATION, AND PRESENTATION MANUAL (JUNE 2007)



RELATIVE ST	RENGTH OF INTACT ROCK
Term	Uniaxial Compressive Strength (PSI)
Extremely Strong	> 30,000
Very Strong	14,500 - 30,000
Strong	7,000 - 14,500
Medium Strong	3,500 - 7,000
Weak	700 - 3,500
Very Weak	150 - 700
Extremely Weak	〈 150

BEDDING	SPACING
Description	Thickness / Spacing
Massive	Greater than 10 ft
Very thickly bedded	3 to 10 ft
Thickly bedded	1 to 3 ft
Moderately bedded	3-5/8" to 1 ft
Thinly bedded	1-1/4" to 3-5/8"
Very thinly bedded	3/8" to 1-1/4"
Laminated	Less than 3/8"

REGISTERED CIVIL ENGINEER DATE OR OFESSIONAL PLANS APPROVAL DATE ROUTE KILOMETER POST SHEET TOTAL SHEETS No SHEETS NO SHEET TOTAL SHEETS NO SHEETS OR OFESSIONAL OR OF						
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	ROCK HARDNESS				
Description	Criteria				
Extremely Hard	Specimen cannot be scratched with a pocket knife or sharp pick; can only be chipped with repeated heavy hammer blows.				
Very Hard	Specimen cannot be scratched with a pocket knife or sharp pick. Breaks with repeated heavy hammer blows.				
Hard	Specimen can be scratched with a pocket knife or sharp pick with difficulty (heavy pressure). Heavy hammer blows required to break specimen.				
Moderately Hard	Specimen can be scratched with pocket knife or sharp pick with light or moderate pressure. Core breaks with moderate hammer pressure.				
Moderately Soft	Specimen can be grooved 1/6" deep with a pocket knife or sharp pick with moderate or heavy pressure. Breaks with light hammer blow or heavy manual pressure.				
Soft	Specimen can be grooved or gouged easily by a pocket knife or sharp pick with light pressure, can be scratched with fingernail. Breaks with light to moderate manual pressure.				
Very Soft	Specimen can be readily indented, grooved or gouged with fingernail, or carved with a pocket knife. Breaks with light manual pressure.				

FRACTURE DENSITY					
Description	Observed Fracture Density				
Unfractured	No fractures.				
Very slightly fractured	Lengths greater than 3 feet.				
Slightly fractured	Lengths from 1 to 3 feet with few lengths less than 1 foot or greater than 3 feet.				
Moderately fractured	Lengths mostly in 4" to 1 foot range with most lengths about 8"				
Intensely fractured	Lengths average from 1 to 4" with scattered fragmented intervals with lengths less than 4"				
Very intensely fractured	Mostly chips and fragments with a few scattered short core lengths.				

Combination descriptors (such as "Very intensely to intensely fractured") are used where equal distribution of both fracture density characteristics is present over a significant interval or exposure, or where characteristics are "in between" the descriptor definitions. Only two adjacent descriptors may be combined.

		WEATHERI	NG DESCRIPTORS FOR	INTACT ROCK		
		Diagr	nostic features			
Description Chemical Weathering-Disco and/or oxidation Body of Rock	Chemical Weathering–Discoloration and/or oxidation		Mechanical Weathering- Grain boundary condi- tions (disaggregation)	Texture and Solutioning		General Characteristics
	Fracture Surfaces	primarily for granitics and some coarse-grained sediments	Texture	Solutioning	General Character 151105	
resh	No discoloration, not oxidized.	No discoloration or oxidation.	No separation, intact (tight).	No change.	No solutioning.	Hammer rings when crystalline rocks are struck.
Slightly Weathered	Discoloration or oxidation is limited to surface of, or short distance from, fractures; some feldspar crystals are dull.	Minor to complete discoloration or oxidation of most surfaces.	No visible separation, intact (tight).	Preserved.	Minor leaching of some solu- ble minerals may be noted.	Hammer rings when crystalline rocks are struck. Body of rock not weakened.
Moderately Neathered	Discoloration or oxidation extends from fractures usually throughout; Fe-Mg minerals are "rusty," feldspar crystals are "cloudy."	All fracture surfaces are discolored or oxidized.	Partial separation of boundaries visible.	Generally preserved.	Soluble min- erals may be mostly leached.	Hammer does not ring when rock is struck. Body of rock is slightly weakened.
Intensely Weathered	Discoloration or oxidation throughout; all feldspars and Fe-Mg minerals are altered to clay to some extent; or chemical alteration produces in-situ disaggregation, see grain boundary conditions.	All fracture surfaces are discolored or oxidized, surfaces friable.	Partial separation, rock is friable; in semiarid conditions granitics are disaggregated.	Texture altered by chemical disintegra- tion (hy- dration, argillation).	Leaching of soluble min- erals may be complete.	Dull sound when struck with hammer, usually can be broken with moderate to heavy manual pressure or by light hammer blow without reference to planes of weakness such as incipient or hairline fractures, or veinlets. Rock is significantly weakened.
Decomposed	Discolored or oxidized throughout, but resistant minerals such as quartz may be unaltered; all feldspars and Fe-Mg minerals are completely altered to clay.		Complete separation of grain boundaries (disaggregated).	Resembles a soil, partial or complete remnant rock structure may be preserved; leaching of soluble minerals usually complete.		Can be granulated by hand. Resistant minerals such as quartz may be present as "stringers" or "dikes."

Combination descriptors (such as "slightly weathered to fresh") are permissible where equal distribution of both weathering characteristics is present over significant intervals or where characteristics present are "in between" the diagnostic feature. However, combination descriptors should not be used where significant, identifiable zones can be delineated. Only two adjacent descriptors may be combined. "Very intensely weathered" is the combination descriptor for "intensely weathered to decomposed."

ENGINEERING SERVICES	GEOTECHNICAL SERVICES		DIVISION OF ENGINEERING SERVICES	BRIDGE NO.	ROCK LEGEND
PREPARED BY		CALIFORNIA	STRUCTURE DESIGN	POST MILE	
CHECKED BY		DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH		LOG OF TEST BORINGS
GS LOTB ROCK LEGEND	ORIGINAL SCALE IN INCHES FOR REDUCED PLANS		CU EA	DISREGARD PRI EARLIER REVIS	NTS BEARING REVISION DATES SHEET OF

REFERENCE: CALTRANS SOIL & ROCK LOGGING, CLASSIFICATION, AND PRESENTATION MANUAL (JUNE 2007)

CEMENTATION				
Description	Criteria			
Weak	Crumbles or breaks with handling or little finger pressure.			
Moderate	Crumbles or breaks with considerable finger pressure.			
Strong	Will not crumble or break with finger pressure.			

	BOREHOLE IDENTIFICATION					
Symbol	Hole Type	Description				
Size	А	Auger Boring				
Size	R P	Rotary drilled boring Rotary percussion boring (air)				
Size	R	Rotary drilled diamond core				
Size	HD HA	Hand driven (1-inch soil tube) Hand Auger				
•	D	Dynamic Cone Penetration Boring				
A	CPT	Cone Penetration Test (ASTM D 5778-95)				
r - 1 r - 1	0	Other				
		Note: Size in inches.				

Hole I.D.

Boring Date

ROTARY BORING

Terminated at Elev

Hammer Energy Ratio (ER;) = %

GWS Elev.

Date measured

L Material change

Top Hole El.

Casing driven

SPT N-Value

or as noted

(inches)

Size of Sampler

(per ASTM 1586-99),

P = push sample,

	CONSISTENCY OF COHESIVE SOILS							
Description	Unconfined Compressive Strength (tsf)	Pocket Penetrometer Measurement (tsf)	Torvane Measurement (tsf)	Field Approximation				
Very Soft	< 0 . 25	< 0.25	<pre>Co.12 Easily penetrated several Easily penetrated several Easily penetrated several Easily penetrated several Easily penetrated several</pre>					
Soft	0.25 to 0.50	0.25 to 0.50	0.12 to 0.25	Easily penetrated several inches by thumb				
Medium Stiff	0.50 to 1.0	0.50 to 1.0	0.25 to 0.50	Penetrated several inches by thumb with moderate effort				
Stiff	1 to 2	1 to 2	0.50 to 1.0	Readily indented by thumb but penetrated only with great effort				
Very Stiff	2 to 4	2 to 4	1.0 to 2.0	Readily indented by thumbnail				
Hard	> 4.0	> 4.0	> 2.0	Indented by thumbnail with difficulty				

KILOMETER POST TOTAL PROJECT

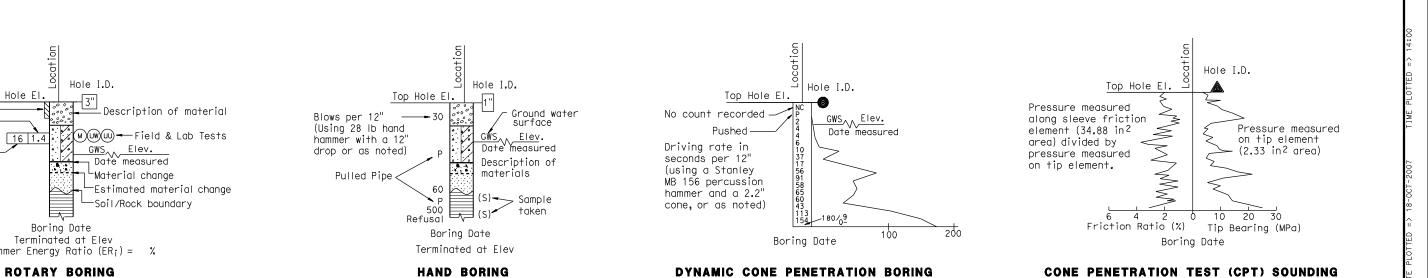
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COUNTY

PLANS APPROVAL DATE

	PLASTICITY OF FINE-GRAINED SOILS				
Description	Criteria				
Nonplastic	A 1/8-inch thread cannot be rolled at any water content.				
Low	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit.				
Medium	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be rerolled after reaching the plastic limit. The lump crumbles when drier than the plastic limit.				
High	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit.				



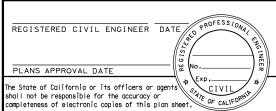
ENGINEERING SERVICES	GEOTECHNICAL SERVICES		DIVISION OF ENGINEERING SERVICES	BRIDGE NO.	SOIL LEGEND
PREPARED I	Υ	CALIFORNIA	STRUCTURE DESIGN	POST MILE	
CHECKED E	r	DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH		LOG OF TEST BORINGS
S LOTB SOIL LEGEND	ORIGINAL SCALE IN INCHES FOR REDUCED PLANS		CU	DISREGARD PRINTS BEARING EARLIER REVISION DATES	REVISION DATES SHEET OF

GROUP SYMBOLS AND NAMES									
	c/Symbol	Group Names	Graphi	Graphic/Symbol Group Names					
	GW GP	Well-graded GRAVEL Well-graded GRAVEL with SAND Poorly graded GRAVEL Poorly graded GRAVEL with SAND		CL	Lean CLAY Lean CLAY with SAND Lean CLAY with GRAVEL SANDY lean CLAY SANDY lean CLAY with GRAVEL GRAVELLY lean CLAY				
	GW-GM	Well-graded GRAVEL with SILT Well-graded GRAVEL with SILT and SAND		CL-ML	GRAVELLY lean CLAY with SAND SILTY CLAY SILTY CLAY with SAND SILTY CLAY with GRAVEL SANDY SILTY CLAY				
	GW-GC	Well-graded GRAVEL with CLAY (or SILTY CLAY) Well-graded GRAVEL with CLAY and SAND (or SILTY CLAY and SAND)		CL IVIL	SANDY SILTY CLAY with GRAVEL GRAVELLY SILTY CLAY GRAVELLY SILTY CLAY with SAND				
	GP-GM	Poorly graded GRAVEL with SILT Poorly graded GRAVEL with SILT and SAND	-	ML	SILT SILT with SAND SILT with GRAVEL SANDY SILT				
0000	GP-GC	Poorly graded GRAVEL with CLAY (or SILTY CLAY) Poorly graded GRAVEL with CLAY and SAND (or SILTY CLAY and SAND)			SANDY SILT with GRAVEL GRAVELLY SILT GRAVELLY SILT with SAND				
	GM	SILTY GRAVEL SILTY GRAVEL with SAND		OL	ORGANIC lean CLAY ORGANIC lean CLAY with SAND ORGANIC lean CLAY with GRAVEL SANDY ORGANIC lean CLAY				
	GC	CLAYEY GRAVEL CLAYEY GRAVEL with SAND			SANDY ORGANIC lean CLAY with GRAVEL GRAVELLY ORGANIC lean CLAY GRAVELLY ORGANIC lean CLAY with SAND				
460	GC-GM	SILTY, CLAYEY GRAVEL SILTY, CLAYEY GRAVEL with SAND		OL	ORGANIC SILT ORGANIC SILT with SAND ORGANIC SILT with GRAVEL SANDY ORGANIC SILT				
	SW	Well-graded SAND Well-graded SAND with GRAVEL			SANDY ORGANIC SILT with GRAVEL GRAVELLY ORGANIC SILT GRAVELLY ORGANIC SILT with SAND Fat CLAY				
	SP SW-SM	Poorly graded SAND Poorly graded SAND with GRAVEL Well-graded SAND with SILT		СН	FAT CLAY FAT CLAY WITH SAND FAT CLAY WITH GRAVEL SANDY FAT CLAY SANDY FAT CLAY WITH GRAVEL GRAVELLY FAT CLAY				
	SW-SC	Well-graded SAND with SILT and GRAVEL Well-graded SAND with CLAY (or SILTY CLAY) Well-graded SAND with CLAY and GRAVEL			GRAVELLY fat CLAY with SAND Elastic SILT Elastic SILT with SAND				
A	SP-SM	Well-graded SAND with CLAY and GRAVEL (or SILTY CLAY and GRAVEL) Poorly graded SAND with SILT Poorly graded SAND with SILT and GRAVEL	-	МН	Elastic SILT with GRAVEL SANDY elastic SILT SANDY elastic SILT with GRAVEL GRAVELLY elastic SILT GRAVELLY elastic SILT with SAND				
	SP-SC	Poorly graded SAND with CLAY (or SILTY CLAY) Poorly graded SAND with CLAY and GRAVEL (or SILTY CLAY and GRAVEL)		ОН	ORGANIC fat CLAY ORGANIC fat CLAY with SAND ORGANIC fat CLAY with GRAVEL SANDY ORGANIC fat CLAY				
	SM	SILTY SAND SILTY SAND with GRAVEL			SANDY ORGANIC fat CLAY with GRAVEL GRAVELLY ORGANIC fat CLAY GRAVELLY ORGANIC fat CLAY with SAND				
	SC	CLAYEY SAND with GRAVEL		ОН	ORGANIC elastic SILT ORGANIC elastic SILT with SAND ORGANIC elastic SILT with GRAVEL SANDY ORGANIC elastic SILT				
	SC-SM	SILTY, CLAYEY SAND with GRAVEL			SANDY ORGANIC elastic SILT with GRAVEL GRAVELLY ORGANIC elastic SILT GRAVELLY ORGANIC elastic SILT with SAND				
77 77 7 77 77 7	РТ	PEAT	J J J J J J J J J J J	OL/OH	ORGANIC SOIL ORGANIC SOIL with SAND ORGANIC SOIL with GRAVEL SANDY ORGANIC SOIL				
		COBBLES COBBLES and BOULDERS BOULDERS	J		SANDY ORGANIC SOIL with GRAVEL GRAVELLY ORGANIC SOIL GRAVELLY ORGANIC SOIL with SAND				

FIELD	AND	LABOR	RATORY
	TE	STING	

- (C) Consolidation (ASTM D 2435)
- (CL) Collapse Potential (ASTM D 5333)
- (CP) Compaction Curve (CTM 216)
- CR Corrosivity Testing (CTM 643, CTM 422, CTM 417)
- COD Consolidated Undrained Triaxial (ASTM D 4767)
- DS) Direct Shear (ASTM D 3080)
- EI) Expansion Index (ASTM D 4829)
- (M) Moisture Content (ASTM D 2216)
- OC) Organic Content-% (ASTM D 2974)
- P Permeability (CTM 220)
- (PA) Particle Size Analysis (ASTM D 422)
- Pl Plasticity Index (AASHTO T 90) Liquid Limit (AASHTO T 89)
- (PL) Point Load Index (ASTM D 5731)
- (PM) Pressure Meter
- (PP) Pocket Penetrometer
- (R) R-Value (CTM 301)
- (SE) Sand Equivalent (CTM 217)
- (SG) Specific Gravity (AASHTO T 100)
- (SL) Shrinkage Limit (ASTM D 427)
- (SW) Swell Potential (ASTM D 4546)
- (TV) Pocket Torvane
- Unconfined Compression-Soil
 (ASTM D 2166)
 Unconfined Compression-Rock
 (ASTM D 2938)
- Unconsolidated Undrained Triaxial (ASTM D 2850)
- (UW) Unit Weight (ASTM D 4767)
- (VS) Vane Shear (AASHTO T 223)

DIST	COUNTY	ROUTE	KILOMETER POST TOTAL PROJECT	SHEET No	TOTAL SHEETS



APPARENT DENSIT	TY OF COHESIONLESS SOILS				
Description	SPT N ₆₀ (Blows / 12 inches)				
Very loose	0 - 4				
Loose	5 - 10				
Medium Dense	11 - 30				
Dense	31 - 50				
Very Dense	> 50				

MOISTURE							
Description Criteria							
Dry	Absence of moisture, dusty, dry to the touch						
Moist	Damp but no visible water						
Wet	Visible free water, usually soil is below water table						

PERCE	NT OR PROPORTION OF SOILS
Description	Criteria
Trace	Particles are present but estimated to be less than 5%
Few	5 to 10%
Little	15 to 25%
Some	30 to 45%
Mostly	50 to 100%

PARTICLE SIZE								
Description		Size						
Boulder		> 12"						
Cobble		3" to 12"						
Gravel	Coarse 3/4" to 3"							
Graver	Fine	No. 4 to 3/4"						
	Coarse	No. 10 to No. 4						
Sand	Medium	No. 40 to No. 10						
	Fine	No. 200 to No. 40						

ENGINEERING SERVICES	GEOTECHNICAL SERVICES		A I E OF DIVISION OF ENGINEERING SERVICES		BRIDGE NO. SOIL			LEGEND		
PREPARED BY		CALIFORNIA		POST MILE	1 4	OG OF				
CHECKED BY		DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH		<u>L</u>	Ju or			NUS	
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	•				·		DIST COUNTY	ROUTE POST MILES TOTAL PROJECT	SHEET TOTAL No SHEETS
									RED GEOLO
							CERTIFIED ENGINEE	ERING GEOLOGIST	KED GEOLOGIA
								// No	
							PLANS APPROVAL	L DATE Exp	CERTIFIED /
							The State of Californi shall not be responsit	id or its officers or agents ble for the accuracy or ronic copies of this plan sheet.	GEOLOGIST CRIMIT
							completeness of electr	ble for the accuracy or ronic copies of this plan sheet.	OF CALIFOR
ENGINEERING SERVI	CES GE	OTECHNICAL SERV	VICES	STATE OF	DIVISION OF ENGINEERING SERVICES	BRIDGE NO.			
FUNCTIONAL SUPERVISOR	DRAWN BY:	FIELD INVESTIGATION BY:		CALIFORNIA	STRUCTURE DESIGN	Deer Wiles			
NAME:	CHECKED BY:	1		DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH	POST MILES	LOG OF TE	ST BORINGS	;
		1	ORIGINAL SCALE IN INCHES FOR REDUCED PLANS		CU EA	DISREGARD PRINTS BEARING EARLIER REVISION DATES		REVISION DATES	SHEET OF
OGS CIVIL LOG OF TEST BORINGS SHEET						I HARLIER REVISION DATES			
			FOR REDUCED PLANS	0 1 2 3	FILE => LOTB border.dgn	EARCIEN NEVISION DATES	*		